

13.122 Lecture 1

Primary Load: Bending Moment and Shear Force

Introduction to course:

Design process
Structural design process

General course content:

13.122 Ship Structural Design

- A. Loads on ship/offshore platforms
 - Calculation of loads
 - buoyancy, shear, bending moment
 - "hand" using excel
- B. Review of bending, shear and torsion - open sections
- C. Modeling a structure
 - Maestro
 - checking loads and moments
- D. Development of limit states and failure modes
 - stress analysis of ship/ocean system structure
- E. Design of section for bending
 - project
- F. Matrix analysis (Grillage), FEM Introduction

Expected outcome: an ability to effectively use structural design tools with an understanding of the underlying analysis.

Changes from previous years:

- reduced 13.10 review (2002)
- calculation of loads (bending moment and shear force) (2001)
- introduce Maestro earlier for modeling and load analysis (2001)
- introduce open section torsion
- revert to earlier problem sets for mathcad limit state analysis (2001)

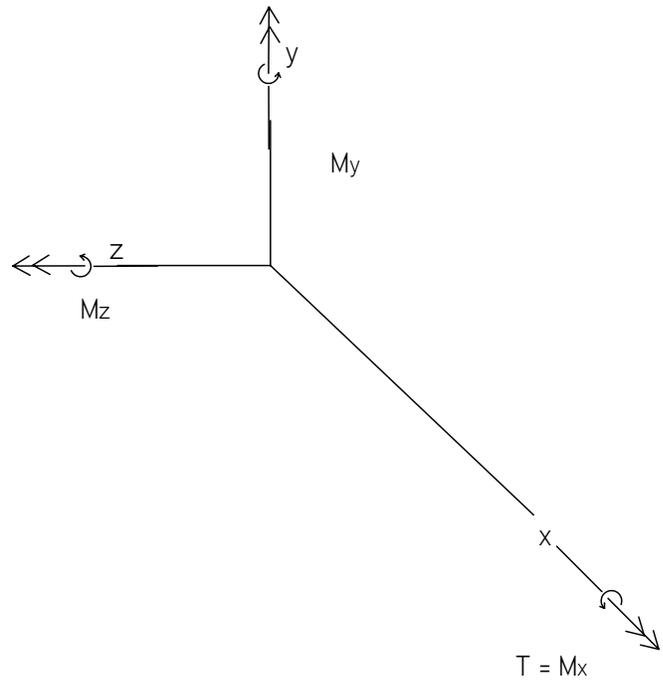
Sign Convention

In general, we will use "structural" sign convention described in Shames: 10.2 page 286

An axial force or bending moment acting on a beam cross-section is positive if it acts on a positive face and is directed in a positive coordinate direction. The shear force is positive if it acts in the *negative* direction on a *positive* face.

(positive face defined by outward normal in positive coordinate direction)

Moment of inertia is defined relative to the axis for measuring distance: $I_z = \int y^2 dA$; $I_y = \int z^2 dA$



$\sigma_x = \tau_{xx}$ $\tau_{xy} = \tau_{yx}$ etc.

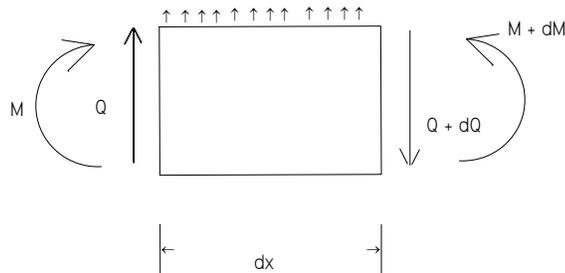
$$\sigma_x = \frac{-M_z \cdot y}{I_z}$$

$$\sigma_x = \frac{M_y \cdot z}{I_y} \quad \text{later}$$

define f = load, positive in +y direction,

$$dQ = f \cdot dx$$

$$Q = \int_0^x f(x) dx + Q(x=0) = \int_0^x f(x) dx$$



moments around right face =>

$$M + Q \cdot dx + f(x) \cdot dx \cdot \frac{dx}{2} - M + dM = 0 \quad \Rightarrow \quad Q = \frac{d}{dx} M$$

$$M(x) = \int_0^x Q(x) dx + M(x=0) = \int_0^x Q(x) dx$$

Part 1: Calculation of loads

buoyancy, shear, bending moment

- a) shear and bending moment from distributed force per length
- b) shear and bending moment from point force
- c) algorithms for calculation

a) shear and bending moment from distributed force per length

suppose we have a distribution of weight per foot w , constant over a distance $\xi_0 \Rightarrow \xi_1$

mathcad has an expression that handles the $<$ and $>$ relationships:
 $a > b = 1$ if true, 0 if false

$$a := 1 \quad b := 2 \quad a > b = 0$$

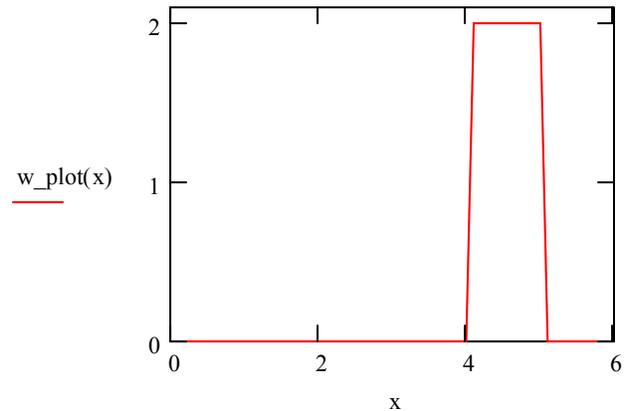
$$b > a = 1$$

for example: for $i := 3$ and $x := 0, 0.1..6$

weight per foot between $\xi_{i,0} \Rightarrow \xi_{i,1}$

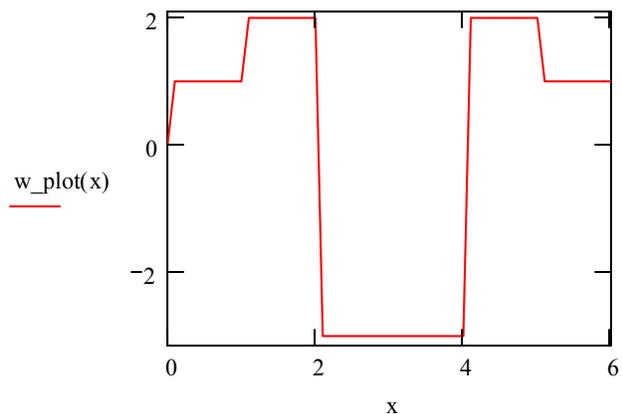
$$w := \begin{pmatrix} 1 \\ 2 \\ -3 \\ 2 \\ 1 \end{pmatrix} \quad \xi := \begin{pmatrix} 0 & 1 \\ 1 & 2 \\ 2 & 4 \\ 4 & 5 \\ 5 & 6 \end{pmatrix}$$

$$w_plot(x) := w_i \cdot (\xi_{i,0} < x \leq \xi_{i,1})$$



by superposition: for $i := 0..4$

$$w_plot(x) := \sum_{i=0}^4 w_i \cdot (\xi_{i,0} < x \leq \xi_{i,1})$$



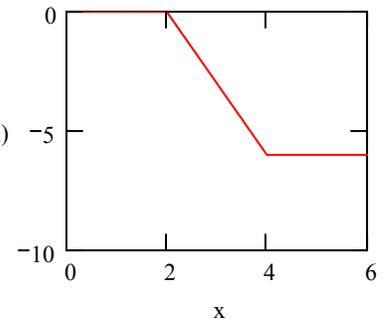
what is shear force from w_i ?

$$\text{shear}(x) := \int_0^x w_i(\xi) d\xi = \begin{cases} 0 & \xi_{i,0} > x \\ w_i \cdot (x - \xi_{i,0}) & \xi_{i,0} < x \leq \xi_{i,1} \\ w_i \cdot (\xi_{i,1} - \xi_{i,0}) & x > \xi_{i,1} \end{cases}$$

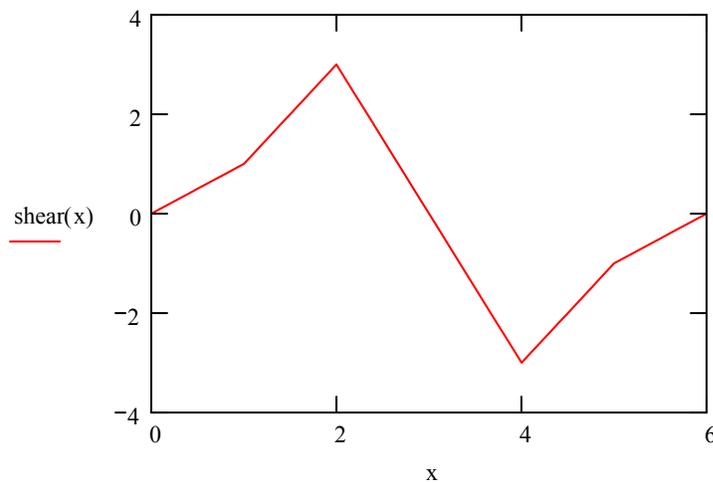
for example: $i := 2$

$$\text{shear}(x) := w_i \cdot (x - \xi_{i,0}) \cdot (\xi_{i,0} < x \leq \xi_{i,1}) + w_i \cdot (\xi_{i,1} - \xi_{i,0}) \cdot (x > \xi_{i,1})$$

again by superposition: $i := 0..4$ using
lower limit = $ll := 0$ and upper limit = $ul := 4$



$$\text{shear}(x) := \sum_{i=ll}^{ul} \left[w_i \cdot (x - \xi_{i,0}) \cdot (\xi_{i,0} < x \leq \xi_{i,1}) + w_i \cdot (\xi_{i,1} - \xi_{i,0}) \cdot (x > \xi_{i,1}) \right]$$



for later comparison, let's rename this $\text{shear}_1(x) := \text{shear}(x)$

note that shear curve starts and ends at 0; why??

$$\sum_i w_i \cdot (\xi_{i,1} - \xi_{i,0}) = 0$$

now what is bending moment from weight per foot as above?

$$\text{bending_moment}(x) := \int_0^x \text{shear}(\xi) d\xi =$$

$$\int_0^x \left[w_i \cdot (x - \xi_{i,0}) \cdot (\xi_{i,0} < x \leq \xi_{i,1}) + w_i \cdot (\xi_{i,1} - \xi_{i,0}) \cdot (x > \xi_{i,1}) \right] d\xi$$

$$= 0 \text{ when } ; x < \xi_{i,0}$$

$$= w_i \cdot \left[\left(\frac{x^2}{2} - \xi_{i,0} \cdot x \right) - \left[\frac{(\xi_{i,0})^2}{2} - (\xi_{i,0})^2 \right] \right] \text{ when } \xi_{i,0} < x < \xi_{i,1} \text{ or } = \frac{w_i}{2} \cdot (x - \xi_{i,0})^2 \text{ after simplifying}$$

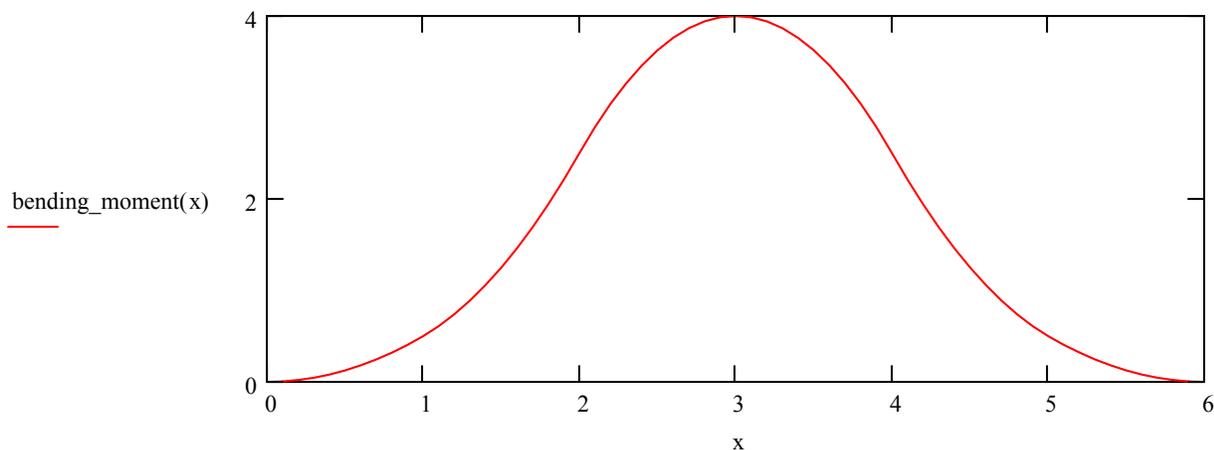
$$= \left[\frac{w_i}{2} \cdot (x - \xi_{i,0})^2 \right] \cdot (\xi_{i,0} < x \leq \xi_{i,1}) \text{ using the notation above.}$$

$$\text{and } = w_i \cdot \left[(\xi_{i,1} - \xi_{i,0}) \cdot (x - \xi_{i,1}) + \frac{1}{2} \cdot (\xi_{i,1} - \xi_{i,0}) \cdot (\xi_{i,1} - \xi_{i,0}) \right] \cdot (x > \xi_{i,1}) \text{ when } x > \xi_{i,1}$$

$$\text{simplifying } \Rightarrow \left(\frac{x^2}{2} - \xi_{i,0} \cdot x \right) - \left[\frac{(\xi_{i,0})^2}{2} - (\xi_{i,0})^2 \right] = \frac{x^2}{2} - \xi_{i,0} \cdot x + \frac{(\xi_{i,0})^2}{2} = \frac{1}{2} \cdot (x - \xi_{i,0})^2$$

therefore; bending moment (x) = for lower limit: ll:= 0 and upper limit = ul:= 4

$$\text{bending_moment}(x) := \sum_{i=ll}^{ul} \left[w_i \cdot \left[\frac{1}{2} \cdot (x - \xi_{i,0})^2 \right] \cdot (\xi_{i,0} < x \leq \xi_{i,1}) \dots \right. \\ \left. + w_i \cdot \left[(\xi_{i,1} - \xi_{i,0}) \cdot (x - \xi_{i,1}) + \frac{1}{2} \cdot (\xi_{i,1} - \xi_{i,0}) \cdot (\xi_{i,1} - \xi_{i,0}) \right] \cdot (x > \xi_{i,1}) \right]$$



for later comparison we will save this as $\text{bending_moment}_1(x) := \text{bending_moment}(x)$

b) shear and bending moment from point force

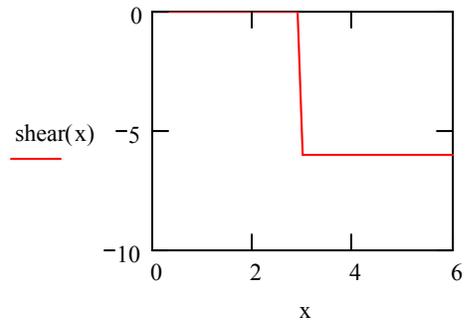
now consider if load is concentrated at a point between $\xi_{i,0}$ and $\xi_{i,1}$;

define $f_i := w_i \cdot (\xi_{i,1} - \xi_{i,0})$ when $x = xx_i$ i. e. and we will define $xx_i := \frac{(\xi_{i,1} + \xi_{i,0})}{2}$

this is not necessary as xx_i can be at any position (but usually between the end points).

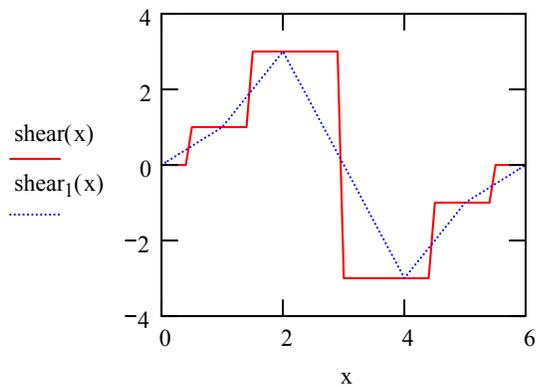
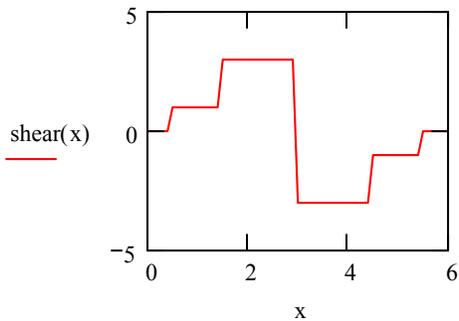
In this case $shear(x) := f_i \cdot (x \geq xx_i)$

for example: $i := 2$ $xx_1 = 3$ $shear(x) := f_1 \cdot (x \geq xx_1)$



as above total shear along beam with $ll := 0$ and $ul := 4 \Rightarrow shear(x) := \sum_{i=ll}^{ul} f_i \cdot (x \geq xx_i)$

let's compare this version with the earlier weight per foot $shear_1(x)$



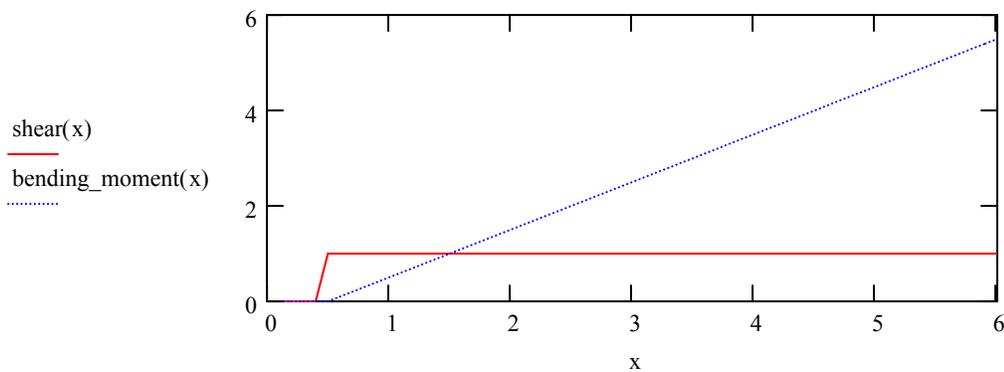
now for bending moment: $\text{bending_moment}(x) := \int_0^x \text{shear}(\xi) d\xi$; as above let's look first at bending moment

from each component: $\text{shear}(x) := f_i \cdot (x \geq xx_i)$

$$\text{bending_moment}(x) := \int_0^x f_i \cdot (x \geq xx_i) d\xi = \text{bending_moment}(x) := f_i \cdot (x - xx_i) \cdot (x \geq xx_i)$$

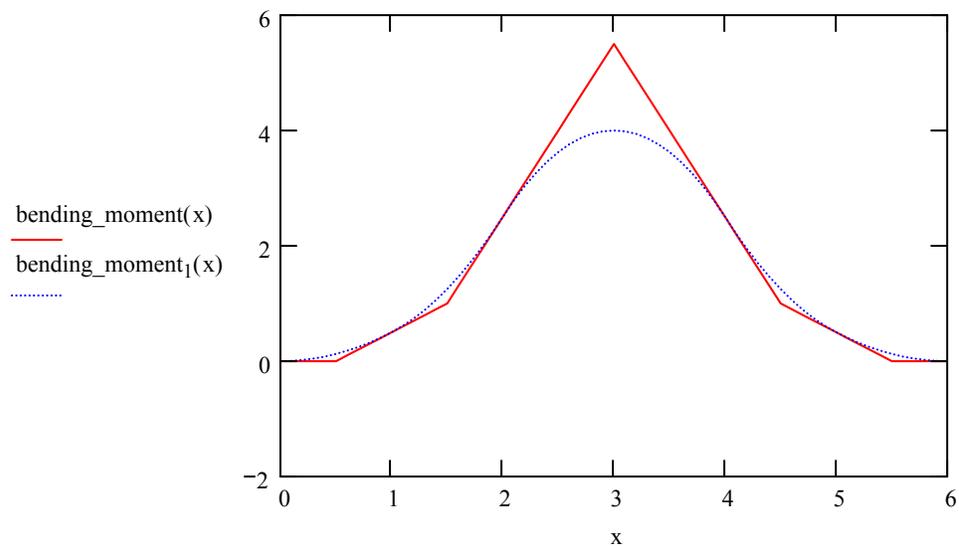
plotting shear and bending moment for $i := 0$; $\text{shear}(x) := f_i \cdot (x \geq xx_i)$ and

$$\text{bending_moment}(x) := f_i \cdot (x - xx_i) \cdot (x \geq xx_i)$$



and as above total bending moment is the superposition of all components:

$$\text{bending_moment}(x) := \sum_{i=1}^{ul} f_i \cdot (x - xx_i) \cdot (x \geq xx_i) \text{ again comparing with the weight per foot from above}$$



this doesn't look all that great, but we let a constant 3 span two intervals, what if we split it into two parts?

weight per foot between $\xi_{i,0} \Rightarrow \xi_{i,1}$

$i := 0..5$

$ll := 0$

$ul := 5$

$$w := \begin{pmatrix} 1 \\ 2 \\ -3 \\ -3 \\ 2 \\ 1 \end{pmatrix}$$

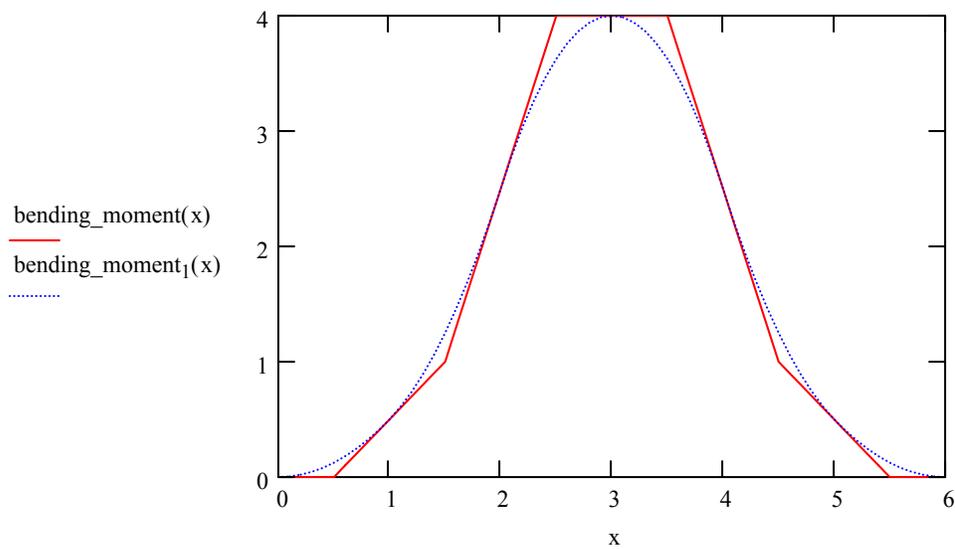
$$\xi := \begin{pmatrix} 0 & 1 \\ 1 & 2 \\ 2 & 3 \\ 3 & 4 \\ 4 & 5 \\ 5 & 6 \end{pmatrix}$$

$$xx_i := \frac{(\xi_{i,1} + \xi_{i,0})}{2}$$

$$xx = \begin{pmatrix} 0.5 \\ 1.5 \\ 2.5 \\ 3.5 \\ 4.5 \\ 5.5 \end{pmatrix}$$

$$\text{bending_moment}(x) := \sum_{i=ll}^{ul} f_i \cdot (x - xx_i) \cdot (x \geq xx_i)$$

$$f_i := w_i \cdot (\xi_{i,1} - \xi_{i,0})$$



c) algorithms for calculation

problem: distribution of point loads along length, located at ~midpoint between x_i and x_{i+1}

$nsta := 7$

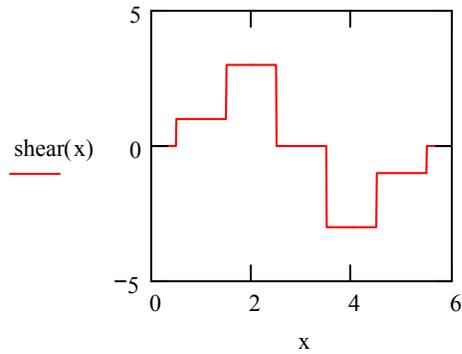
$ul := nsta - 1 \quad ll := 0$

$j := 0..ul - 1$

$$f := \begin{pmatrix} 1 \\ 2 \\ -3 \\ -3 \\ 2 \\ 1 \end{pmatrix} \quad x_s := \begin{pmatrix} 0 \\ 1 \\ 2 \\ 3 \\ 4 \\ 5 \\ 6 \end{pmatrix} \quad xx_j := \frac{x_{s_j} + x_{s_{j+1}}}{2} \quad xx = \begin{pmatrix} 0.5 \\ 1.5 \\ 2.5 \\ 3.5 \\ 4.5 \\ 5.5 \end{pmatrix}$$

$x := x_{s_0}, x_{s_0} + 0.01 .. x_{s_{ul}}$

$$shear(x) := \sum_{i=ll}^{ul-1} f_i \cdot (x \geq xx_i)$$



can represent by values at endpoint of stations: eliminates $(x > x_s)$ term

$s_0 := 0$

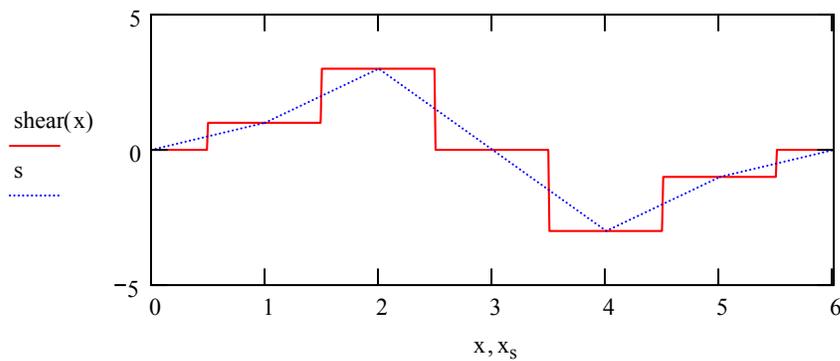
$i := 0..ul - 1$

$s_{i+1} := s_i + f_i$

$s = \begin{pmatrix} 0 \\ 1 \\ 3 \\ 0 \\ -3 \\ -1 \\ 0 \end{pmatrix}$

location

$x_s = \begin{pmatrix} 0 \\ 1 \\ 2 \\ 3 \\ 4 \\ 5 \\ 6 \end{pmatrix}$



bending moment algorithm: as above

$$bm = \int_0^x \text{shear}(x) dx$$

$$bm(x = x_{s_i}) = \int_0^x \text{shear}(x) dx = \int_0^{x_{s_{i-1}}} \text{shear}(x) dx + \int_{x_{s_{i-1}}}^{x_{s_i}} \text{shear}(x) dx = bm(x = x_{s_{i-1}}) + \int_{x_{s_{i-1}}}^{x_{s_i}} \text{shear}(x) dx$$

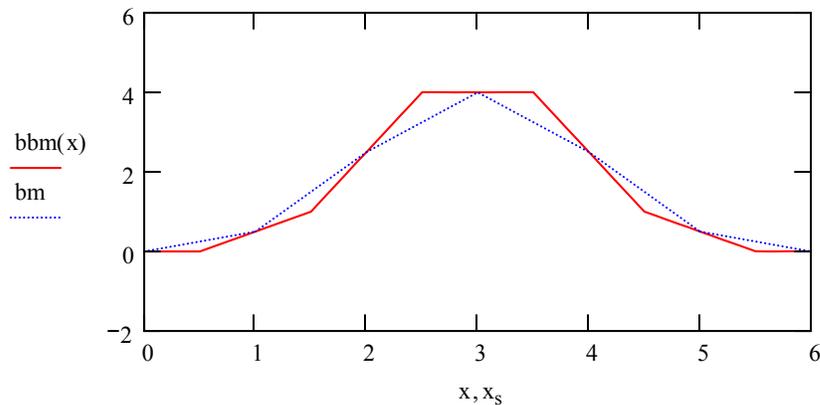
either model for shear =>

$$\int_{x_{s_{i-1}}}^{x_{s_i}} \text{shear}(x) dx = (\text{shear}(x_{s_i}) + \text{shear}(x_{s_{i-1}})) \cdot \frac{(x_{s_i} - x_{s_{i-1}})}{2} = \left(\frac{s_{i-1} + s_i}{2} \right) \cdot (x_{s_i} - x_{s_{i-1}})$$

=> in index form:

$$bm_0 := 0 \quad i := 1..6 \quad bm_i := bm_{i-1} + \left(\frac{s_{i-1} + s_i}{2} \right) \cdot (x_{s_i} - x_{s_{i-1}})$$

$$x := x_{s_0}, x_{s_0} + 0.1 .. x_{s_{ul}} \quad bbm(x) := \int_0^x \text{shear}(x) dx \quad \text{for comparison}$$



this algorithm is amenable to iterative schemes such as could be done in excel:

load	station location	shear force	bending moment
	0	0	0
1			
	1	1	0.5
2			
	2	3	2.5
-3			
	3	0	4
-3			
	4	-3	2.5
2			
	5	-1	0.5
1			
	6	0	0

$$s = \begin{pmatrix} 0 \\ 1 \\ 3 \\ 0 \\ -3 \\ -1 \\ 0 \end{pmatrix} \quad \text{bm} = \begin{pmatrix} 0 \\ 0.5 \\ 2.5 \\ 4 \\ 2.5 \\ 0.5 \\ 0 \end{pmatrix}$$

next we will develop the load from weight-buoyancy