AS ILLUSTRATED IN THE FIRST FEW SLIDES, REGULAR TIME-HARMONIC PLANE PROGRESSIVE WAVES ARE THE FUNDAMENTAL BUILDING BLOCK IN DESCRIBING THE PROPAGATION OF SURFACE WAVE DISTURBANCES IN A DETERMINISTIC & STOCHASTIC SETTING AND IN PREDICTING THE RESPONSE OF FLOATING STRUCTURES IN A SEASTATE.

THEY SATISFY THE BOUNDARY-VALUE PROBLEM!

$$\int \frac{\partial^2 \phi_1}{\partial t^2} + 9 \frac{\partial \phi_1}{\partial z} = 0, \quad z = 0$$

$$\int \frac{\partial \phi_1}{\partial t^2} + 9 \frac{\partial \phi_1}{\partial z} = 0, \quad z = -H$$

THE LAST CONDITION IMPOSES A ZERO NORMAL FLUX CONDITION ACROSS A SEAFLOUR OF CONSTANT DEPTH, H. THE RELATION BETWEEN W AND K IS KNOWN AS THE DISPERSION RELATION OFTEN WRITTEN IN THE FORM:

F(K) DEPENDS ON THE PHYSICS OF THE WAVE PROPAGATION PROBLEM UNDER STUDY.

IN THE CASE OF SOUND PROPAGATION;

WHERE C IS THE SPEED OF SOUND.

THE ABOVE RELATION SUGGESTS THAT ALL

SOUND WAVES IN A MEDIUM WITH ISOTROPIC

PROPERTIES PROPAGATE WITH THE SAME

PHASE SPEED REGADLESS OF THEIR

FREQUENCY OR WAVELENGTH. THESE

ARE KNOWN TO BE NON-DISPERSIVE WAVES

SURFACE WAVES ARE DISPERSIVE SINCE FIX) IS A NONLINEAR FUNCTION OF K. IT WILL BE DERIVED BELOW. BY DEFINITION, REGULAR PLANE PROGRESSINE WAVES ARE SUCH THAT THEIR FREE SURFACE ELEVATION IS DEFINED <u>APRIORI</u> AS FOLLOWS:

$$J(x,t) = A \cos(\omega t - kx)$$
  
=  $A Re \left\{ e^{-ikx + i\omega t} \right\}$ 

OR SCHEMATICALLY:

$$C = PHASE VELOCITY (CREST SPEED)$$

$$T = \frac{\lambda}{C}$$

$$\lambda = 2\pi/C$$

WHERE A IS AN A PRIORI KNOWN WAVE

AMPLITUDE AND THE WAVE FREQUENCY AND

WAVE NUMBER (W, K) PAIR HAVE THE SAME

DEFINITIONS AS IN ALL WAVE PROPAGATION

PROBLEMS, NAMELY:

T = 
$$\frac{2\pi}{\omega}$$
 = -WAVE PERIOD

$$\Delta = \frac{2\pi}{k} = \text{WAVE LENGTH}$$

$$C = \frac{\omega}{k} = \text{WAVE PHASE VELOCITY}$$

BY VIRTUE OF THE DEFINITION OF THE WAVE ELEVATION OF A PLANE PROGRESSIVE REGULAR WAVE, WE SEEK A COMPATIBLE DEFINITION OF THE RESPECTIVE VELOCITY POTENTIAL. LET:

THE PROBLEM REDUCES TO THE DEFINITION

OF  $\psi(x,z)$  AND THE DERIVATION OF THE

APPROPRIATE DISPERSION RELATION BETWEEN

WAND K SO THAT THE LINEAR BONNDARY

VALUE PROBLEM IS SATISFIED.

BEFORE PROCEEDING WITH THE ALGEBRA,
CERTAIN UNDERLYING PRINCIPLES ARE
ALWAYS AT WORK:

● LINEAR SYSTEM THEORY STATES THAT WHEN THE INPUTSIQUAL IS @ iwt THE OUTPUTSIQUAL MUST ALSO BE HARMONIC AND WITH THE SAME FRECQUENCY.

- EXISTS, OTHERWISE THE STATEMENT OF
  THE PHYSICAL AND FOR MATHEMATICAL
  BOUNDARY VALUE PROBLEM 15 FLAWED.
  IF WE CAN FIND A SOLUTION IN MOST
  CASES IT IS THE SOLUTION. SO SIMPLY
  TRY OUT SOLUTIONS THAT MAY MAKE
  SENSE FROM THE PHYSICAL POINT OF VIEW.
- SATISFIED BY A COMPLEX VELOCITY

  POTENTIAL THEN IT IS ALSO SATISFIED

  BY ITS REAL AND IMAGINARY PARTS

IN OUR CASE WE WILL FIRST DERIVE

THE BOUNDARY VALUE PROBLEM SATISFIED

BY THE COMPLEX POTENTIAL (X, Z)

AND THEN WE WILL TRY THE PLAUSIBLE

REPRESENTATION:

TO:

$$\begin{cases} -\omega^{2} \varphi + g \varphi_{z} = 0, \quad Z = 0 \\ \nabla^{2} \varphi = \varphi_{xx} + \varphi_{zz} = 0, \quad -14 \leq 2 \leq 0 \end{cases}$$

$$\varphi_{z} = 0, \quad Z = -14$$

ALLOWING FOR :

$$\varphi(x,z) = \psi(z) e^{-ikx}$$

IT FOLLOWS THAT Y (2) IS SUBJECT TO:

$$\begin{cases} -\omega^{2}\psi + 9\psi_{z} = 0, \quad Z = 0 \\ \psi_{zz} - K^{2}\psi = 0, \quad -H < Z < 0 \\ \psi_{z} = 0, \quad Z = -H \end{cases}$$

VERIFY BY SIMPLE SUBSTITUTION THAT:

SATISFIES THE FIELD EQUATION  $\psi_{22}-k^2\psi=0$ ,

THE SEAFLOOR CONDITION  $\psi_{2}=0$ , Z=-H AND

THE FREE SURFACE CONDITION, ONLY WHEN

$$\omega^2 = g K \tanh KH$$
or 
$$\omega = \{g K \tanh KH\}^{1/2}$$

$$f(K)$$

SO BY ENFORCING THE FREE SURFACE CONPITION WE HAVE DEPINED THE DISPERSION RELATION FOR REGULAR WAVES IN FINITE DEPTH.

RECALL THAT! 
$$\cosh x = \frac{e^{x} + e^{-x}}{2}$$

$$\sinh x = \frac{e^{x} - e^{-x}}{2}$$

$$+ auh x = \sinh x / \cosh x$$

THE RESULTING PLANE PROGRESSIVE WAVE VELOCITY POTENTIAL TAKES THE FORM:

$$\phi(x,z,t) = A \operatorname{Re} \left\{ \frac{ig}{\omega} \frac{\cosh k(z+it)}{\cosh kt} \right\}$$

$$\times e^{-ikx+i\omega t}$$

VERIFY THAT UPON SUBSTITUTION:

$$S_{1} = A \mathbb{R}e \left\{ e^{-ikx+i\omega t} \right\}$$

$$= -\frac{1}{9} \frac{\partial \phi_{1}}{\partial t} \Big|_{z=0}$$

THE CORRESPONDING FLOW VELOCITY AT SOME

POINT X = (x, 2) IN THE FLUID DO HAIN OR ON Z = 0, Z = -H IS SIMPLY GIVEN BY:

$$\vec{\nabla} = \nabla \phi_1$$

THE LINEAR HYDRODYNAMIC PRESSURE DUE TO THE PLANE PROGRESSIVE WAVE, WHICH MUST BE ADDED TO THE HYDROSTATIC, IS

$$P_{1} = -\rho \frac{\partial \phi_{1}}{\partial t}$$

$$= \mathbb{R}e \left\{ \rho g A \frac{\cosh k(2+H)}{\cosh k H} \right\}$$

$$= -ikx + i\omega t \left\{ \right\}$$

FROM THE LAGRANGIAN KINEMATIC RECATION

$$\frac{d\vec{\xi}_{i}}{dt} = \vec{\nabla}(\vec{\xi}_{i}, t) = \nabla \phi_{i}(\vec{\xi}_{i}, t)$$

WE MAY OBTAIN ORDINARY DIFFERENTIAL EQUATIONS GOVERNING 3, (+). MARKING A PATICULAR PARTICLE WITH THE FLUID AT REST, SO THAT \$ (0) = X, WE MAY WRITE:

$$\frac{1}{\xi_i}(t) = \vec{X} + \underbrace{\Delta \xi(t)}_{O(E)}$$

WHERE X IS THE PARTICLE POSITION AT REST AND DE IS ITS DISPLACEMENT DUE TO THE "ARRIVAL" OF A PLANE PROGRESSIVE WAVE. UPON SUBSTITUTION IN THE EQUATION OF MOTION

$$\frac{d \vec{\Delta \xi}}{dt} = \frac{70}{0(\epsilon)} (\vec{x} + \vec{\Delta \xi}, t)$$
SINCE  $\frac{d\vec{x}}{dt} = 0$ . KEEPING TERMS OF  $O(\epsilon)$ 

ON BOTH SIDES, IT FOLLOWS THAT

$$\frac{d \vec{\Delta s}}{dt} = \nabla \phi, (\vec{x}, t) + o(\epsilon^2)$$

THIS EQUATION WHEN FORCED BY THE

VELOCITY VECTOR THAT CORRESPONDS TO

THE PLANE PROGRESSIVE WAVE SOLUTION

DERIVED ABOVE, LEADS TO A HARMONIC

SOLUTION FOR THE PARTICLE DISPLACED

TRAJECTORIES  $\Delta \S(t) = (\Delta \S_1, \Delta \S_3)$  WHICH

ARE CIRCULAR [LEFT AS AN EXERCISE].

IF SECOND-ORDER EFFECTS ARE INCLUDED,
THE PARTICLES UNDER A PLANE PROGRESSIVE
WAVES ALSO UNDERGO A STEADY-STATE
DRIFT KNOWN AS THE STOKES DRIFT.
IT CAN BE EASILY MODELED BASED ON
THE APPROACH DESCRIBED ABOVE
BY SUBSTITUTING SE COND-ORDER
EFFECTS CONSISTENTLY INTO THE
RIGHT-HAND SIDE OF THE EQUATION
OF MOTION (SEEMH).

## DISPERSION RELATION IN DEEP ?

## SHALLOW WATERS

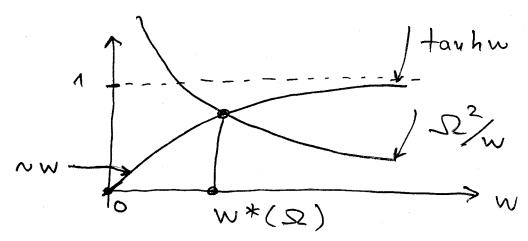
IN FINITE DEPTH :

or 
$$\Omega^2 = \frac{\omega^2 H}{g} = \frac{KH}{W} + \frac{\Delta u h}{W} + \frac{KH}{W}$$

THIS IS A NONLINEAR ALGEBRAIC EQUATION

FOR W = KH WHICH HAS A UNIQUE SOCUTION

AS CAN BE SHOWN GRAPHICALLY



HONOTONICALLY INCREASING WITH AN ASYMPTOTIC VALUE OF 1

52/2: HYPERBOLA MONOTONICALLY DECREASING UNIQUE REAL ROOT W\*(SZ) CAN ONLY
BE FOUND NUMERICALLY. YET IT ALWAYS
EXISTS AND THE ITERATIVE METHODS THAT
HAY BE IMPLEMENTED ALWAYS CONVERGE
RAPIDLY.

GIVEN 
$$(\omega, H) \rightarrow W^*(\Omega^2 = \frac{\omega^2 H}{g}) \equiv W^* = KH$$
  
 $\Rightarrow \frac{2\pi}{\Delta} H = W^*(\Omega) \Rightarrow \Omega = \frac{2\pi H}{W^*(\Omega)}$ 

30 GIVEN THE WAVE FREQUENCY W -> A

PHASE SPEED: 
$$C = \frac{\omega}{K} = \frac{\omega}{\omega^2 g} = \frac{9}{\omega}$$

OR
$$C = \frac{9}{2\pi/T} = \frac{9T}{2\pi}$$

SO THE SPEED OF THE CREST OF A WAVE WITH PERIOD T=10 secs is APPROXIHATELY 15.6 M OR ABOT 30 KNOTS!

OFTEN WE NEED A QUICK ESTIMATE OF THE LENGTH OF A DEED WATER WAVE THE PERIOD OF WHICH WE CAN MEASURE ACCURATELY WITH A STOPWATCH. WE PROCEED AS FOLLOWS:

$$C = \frac{3}{K} = \frac{7}{T}$$

$$\Rightarrow \frac{3}{T} = \frac{3}{3}$$

$$= \frac{9}{2\pi/T}$$

SPEED IS THE RATIO OF
THE WAVELENGTH OVER
THE PERIOD, OR THE TIME
IT TAKES FOR A CREST
TO TRAVEL THAT DISTANCE

$$\Rightarrow \lambda = \frac{9T^2}{2\pi} \sim T^2 + \frac{1}{2}T^2$$

SO THE WAVELENGTH OF A DEEP WATER WAVE IN M IS APPROXI HATELY THE SQUARE OF ITS PERIOD IS SECONDS PLUS HALF THAT AMOUNT.

SO A WAVE WITH PERIOD T= 10 Sec) IS ABOUT 150 m LONG OR ABOUT 492 ft.

IN THE LIMIT OF SHALLOW WATER; KH->0

tanhkH ~ kH

AND 
$$\omega^2 = gk(kH) \Rightarrow \frac{\omega^2}{k^2} = gH$$

THUS, A CCORDING TO LINEAR THEORY
SHALLOW WATER WAVES BECOME NONDISPERSIVE
AS IS THE CASE WITH A COUSTIC WAVES.

UNFORTUNATELY, NONLINEAR EFFECTS
BECOME IMPORTANT IN THE LIMIT OF
SHALLOW WATER AND MUST BE CAREFULLY
BE TAKEN INTO ACCOUNT. SOLITONS AND
WAVE BREAKING ARE SOME MANIFESTATIONS
OF NONLINEARITY. (SEE MEI).

THE TRANSITION FROM DEEP TO FINITE DEPTH WAVE EFFECTS OCCURS FOR VALUES OF

KHI ST. CHECK THAT:

 $\begin{aligned}
& + anh \pi & \simeq 1 \\
& + and & \pi & \simeq 1
\end{aligned}$   $AND FOR KH = \pi \Rightarrow \frac{2\pi H}{3} = \pi$   $\Rightarrow \frac{H}{3} = \frac{1}{2}$ 

SO FOR H/231/2 OR KH>TO WE
ARE EFFECTIVELY DEALING WITH DEEP WATER.